Statistics of Extremes, traffic jams and natural disasters

Deutsches Zentrum
für Luft- und Raumfahrt e.V.
in der Helmholtz-Gemeinschaft

Vortrag_Januar_2006; Prof Reinhart Kühne

(a)

(b)

(c)

(d)

(a) 20 000 data of daily water level of the river Danube, measured at Nagymaros from 1st of January 1901 [7]. (b) Seasonality of the average daily water for one year [see Eq. (1)]. (c) Probability density distribution $P(h)$ of the water level uctuations h [see Eq. (2)]. Note that the vertical scale is logarithmic. (d) Power spectrum of the detrended time series obtained by the standard FFT method. Dotted lines show two scaling regimes, at low frequencies ($f < 0.05 \text{ day}^{-1}$) the characteristic exponent is 1:20:1, at large frequencies ($f > 0.1 \text{ day}^{-1}$) the exponent value is 3:3 0:1.

Rescaled probability density distribution $P_0 = p_2(h_0)P(r|h_0)$ as a function of the rescaled logarithmic rate of change $r_0 = p_2[r - r(h_0)]/(h_0)$ for the data shown in Fig. 2a. The data approximately collapse upon the universal curve Eq. (5) (thin solid line).

Quelle: Imre M. Janosia; Jason A.C. Gallas: Growth of companies and water-level fluctuations of the river Danube; Physica A 271 (1999) 448-457

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*First Measurements to fundamental diagrams
by Greenshields (1934)*

Speed Density Relation $V(\rho)$

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The first Fundamental Diagram

q - v Diagram from 5 minutes interval

[A9 München –
Hollédau, Zeitraum
27.07.-09.08.2000, i.l.
20160 measurement
values]

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Typical bottleneck situations

Type 1

Type 2

Type 3

Speed vs. Flow on I-10 westbound in 5 minute intervals from 4am to 6pm

Results from Performance Monitoring System on Californias Highways (data from 26000 sensors)

- 600 recurrent bottlenecks = 50% of weekday peak delays,
- 28% additionally peak-period congestion delay is caused by collisions,
- 10% of it accounting for 90% of all collision-induced delay

Total vehicle-hours of travel (left) and sources of congestion (right) during peak periods

Demonstration of two 5 - h - periods on two cross sections of the A9 München - Hollndau

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irt
haft

Definition of Traffic Breakdown

- 1) speed drop: $\Delta v > 15 \text{ km/h}$
- 2) speed after drop: $v_2 < 75 \text{ km/h}$
- 3) minimum traffic flow: $q_1 > 1000 \text{ veh/h}$

temporal ratio of different traffic flow classes (%) zeitlicher Anteil der Verkehrsstärke-Klassen [%]

absolut number of traffic breakdowns

probability of traffic breakdown

Approximation for the breakdown probability function for a two lane section

decomposition into free flow and congested traffic

number of vehicles within the cluster:

n

minimum cluster size:

n_{esc}

balance

$$\dot{P}(n,t) = +q P(n-1,t) - q P(n,t)$$

$$+ \frac{1}{\tau} P(n+1,t) - \frac{1}{\tau} P(n,t)$$

Taylor expansion

$$P(n \pm 1, t) \approx P(n, t) \pm \partial_n P(n, t) + \frac{1}{2} \partial_n^2 P(n, t)$$

into balance equation gives
Fokker Planck equation

$$P(n, t) = -\left(q - \frac{1}{\tau}\right) \partial_n P(n, t) + \frac{1}{2} \left(\cancel{q} + \frac{1}{\tau} \right) \partial_n^2 P(n, t)$$

$\approx \frac{1}{\tau}$

dim. less variables (~supressed)

$$x = \frac{n}{n_{\text{esc}}} \quad \tilde{t} = \frac{1}{\tau n_{\text{esc}}^2} \quad \beta = (\tau q - 1) n_{\text{esc}}$$

$$\dot{P}(x, t) = (\partial_x \Phi' + \partial_x^2) P(x, t)$$

with potential

$$\Phi = -2\beta x$$

$$0 < x < 1$$

traffic state dynamics

$$\dot{x} = -\Phi' + \Gamma(t)$$

$\Gamma(t)$ = fluctuating force

$$\langle \Gamma(t) \rangle = 0 \quad \langle \Gamma(t) \Gamma(t') \rangle = 2\delta(t-t')$$

First passage time

Probability of state n anywhere between 0 and n_{esc} at time t when started with $n=0$ at time $t=0$:

$$W(t) = \int_0^{n_{\text{esc}}} (P(n,t|0,0)) dn$$

Drop of probability of state n anywhere between 0 and n_{esc}

$$-\frac{dW(t)}{dt} = - \int_0^{n_{\text{esc}}} \dot{P}(n,t|0,0) dn$$

$\equiv$ probability that state exceeds $n=n_{\text{esc}}$: $\mathcal{P}(t) = - \int_0^{n_{\text{esc}}} \dot{P}(n,t|0,0) dn$

Inserting Fokker Planck equation: $\dot{P}(n,t|0,0) + \partial_n j(n,t|0,0) = 0$

$$\leadsto \mathcal{P}(t) = - \int_0^{n_{\text{esc}}} \dot{P}(n,t|0,0) dn = j(n,t|0,0) \Big|_0^{n_{\text{esc}}} = j(n_{\text{esc}}, t|0,0)$$

separation $P(x,t) = e^{-\frac{\Phi(x)}{2}} e^{-\lambda t} \varphi(x)$ gives $(-\partial_x^2 + \beta^2) \varphi_\nu(x) = \lambda_\nu \varphi_\nu(x)$ with $(-\beta + \partial_x) \varphi(0) = 0$

ground state $\varphi_0 = \begin{cases} N_0 \sin k_0(x-1) \\ N_0 \sinh \kappa_0(x-1) \end{cases} \quad \lambda_0 = \begin{cases} k_0^2 + \beta^2 \\ -\kappa_0^2 + \beta^2 \end{cases} \quad \varphi(1) = 0$

$$N_0^2 = \frac{2}{1 + \beta/\lambda_0} \quad \beta > -1$$

$$N_0^2 = \frac{2}{-1 - \beta/\lambda_0} \quad \beta < -1$$

excited states $\varphi_\nu = N_\nu \sin k_\nu(x-1) \quad \lambda_\nu = k_\nu^2 + \beta^2 \quad N_\nu^2 = \frac{2}{1 + \beta/\lambda_\nu} \quad \nu = 1, 2, \dots$

eigenvalues

$$k_0 \cot k_0 = -\beta \quad \beta > -1$$

$$\kappa_0 \coth \kappa_0 = -\beta \quad \beta < -1$$

$$k_\nu \cot k_\nu = -\beta \quad \nu = 1, 2, \dots$$

time development of probability distribution

$$P(x,t|0,0) = \delta(x)$$

$$P(x,t|0,0) = e^{\frac{-\Phi(x)}{2} + \frac{\Phi(0)}{2}} \sum_{\nu} \varphi_{\nu}(x) \varphi_{\nu}(0) e^{-\lambda_{\nu} t}$$

$$= \begin{cases} \frac{e^{\frac{-x^2}{4t} + \beta x - \beta^2 t}}{\sqrt{4\pi t}} - \frac{e^{\frac{-(x-2)^2}{4t} + \beta x - \beta^2 t}}{\sqrt{4\pi t}} & \beta > -1 \\ e^{\beta x} N_{\nu}^2 \sinh \kappa_{\nu} (x-1) \sinh(-\kappa_{\nu}) e^{-\lambda_0 t} + \dots & \beta < -1 \end{cases}$$

First passage time distribution (left: density; right: cumulative distribution) as probabilistic interpretation of observing a traffic breakdown during a time interval $(t, t+dt)$ or during the observation time $0 \dots T$

cummulative breakdown probability

$$W(T) = 1 - \int_0^1 dx P(x, T | 0, 0) = \frac{1}{2} (1 - \operatorname{erf}(\frac{1}{2\sqrt{T}} - \beta\sqrt{T})) + \frac{e^{2\beta}}{2} (1 - \operatorname{erf}(\frac{1}{2\sqrt{T}} + \beta\sqrt{T}))$$

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Beispiel einer Streckenbeeinflussungsanlage

[Quelle: Engl, H. und F. Lämmel, Highway Deutschland, 1996]

Comparison of two $q-v$ Diagrams from 5 minutes intervals

[A9 München –
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27.07.-09.08.2000,
d.h. 20160
Messwerte]

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Cummulative breakdown probability distribution as function of the traffic volume q as control parameters for different critical cluster sizes m modelling the influence of traffic control measures on the breakdown probability